

Two-Steps Analysis of Movement of the Kfar-Hanassi Network

Gilad Even-Tzur

Department of Mapping and Geo-Information Engineering
Faculty of Civil and Environmental Engineering
Technion - Israel Institute of Technology

*Surveyors Key Role in Accelerated Development, FIG Working Week 2009
3-8 May 2009, Eilat, Israel*



Satellite Photo of Israel

Sea of Galilee

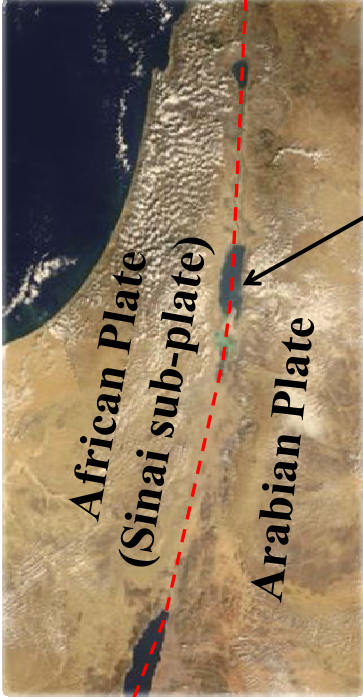
Mediterranean

Dead Sea

we are here
Eilat

Red Sea

Satellite Photo of Israel



The Dead Sea Rift


Displacements of several millimeters per year between the Sinai sub-plate and Arabian plate are expected

African Plate (Sinai sub-plate)

Arabian Plate

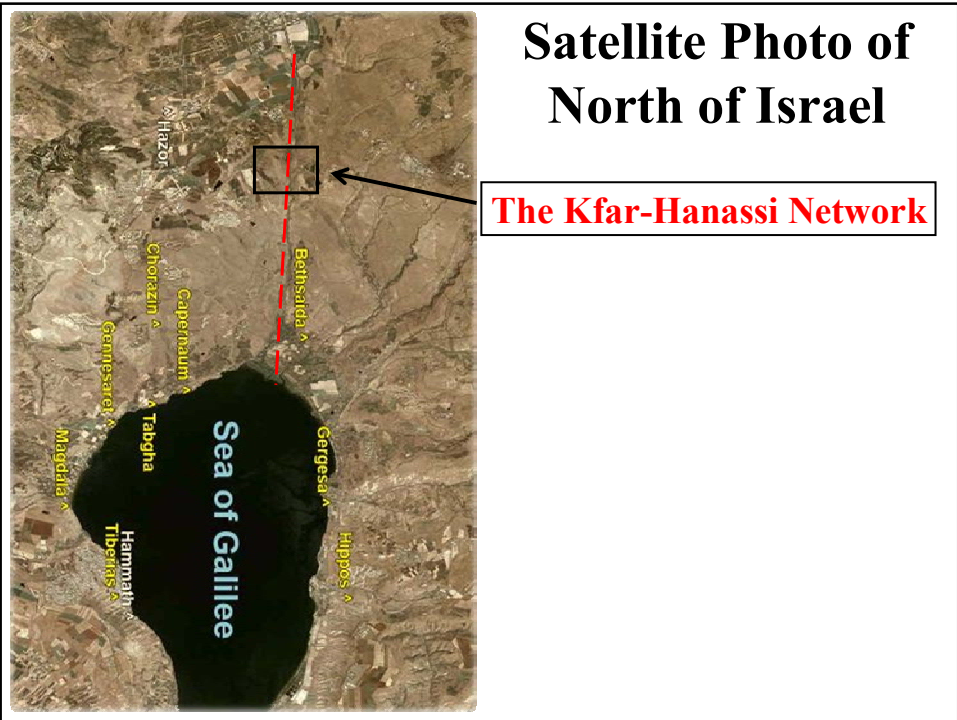
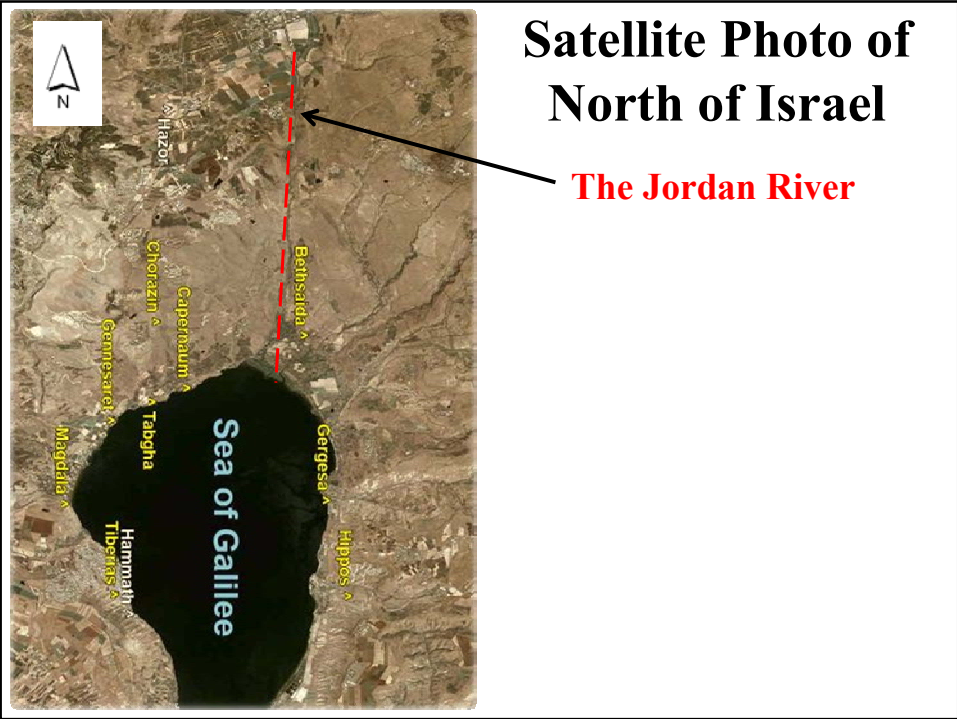
The image is a satellite photograph of the region around the Dead Sea. A vertical red dashed line runs through the center of the Dead Sea, representing the Dead Sea Rift. To the left of this line, the text 'African Plate (Sinai sub-plate)' is written vertically. To the right, 'Arabian Plate' is written vertically. An arrow points from the text 'The Dead Sea Rift' to the red dashed line. Below the image, a paragraph explains that displacements of several millimeters per year are expected between the Sinai sub-plate and the Arabian plate.

Satellite Photo of Israel

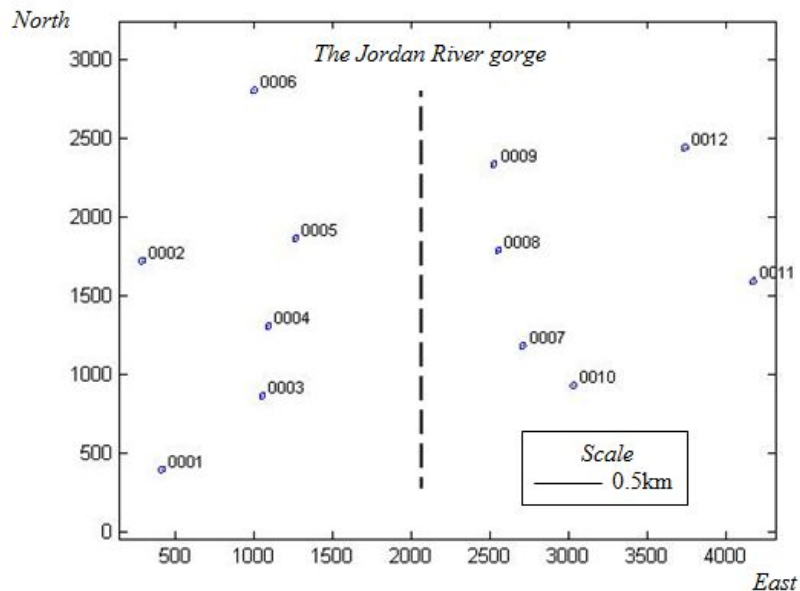


The Kfar-Hanassi Network

The image is a satellite photograph of the same region as the first image. A vertical red dashed line runs through the center of the Dead Sea. A red square is placed on this line, north of the Dead Sea. An arrow points from the text 'The Kfar-Hanassi Network' to this red square.



The Kfar-Hanassi Network



The Kfar-Hanassi Network

- The network consists of twelve benchmarks, six benchmarks on each side of the Jordan River gorge.
- The network spreads over 2.5×4.0 km and was designed for EDM measurements.
- The configuration of the network was mainly affected by the topography of the area and by geomorphological considerations.
- The benchmarks were built according to high technical specifications to ensure their geotechnical stability.

The Benchmarks Specifications

- A borehole, approximately 30cm in diameter, was drilled to a depth of at least 12m.
- The pile consists of concrete reinforcement by steel rods.
- The upper 3m long of the pile is isolated from the ground by a sleeve of asphalt paper.



The Benchmarks Specifications

- Instrument support is provided by three micropiles, 1m deep and 0.2m in diameter with tube sockets for the tripod, driven into the ground around the center pile.



Field Campaigns, 1990-1993

- The network was measured annually during the years 1990-1993 using a Kern Mekometer ME5000.
- 65 distances were measured out of 66.
- Most of the distances were measured twice from each edge of the line.

Field Campaign, 2008

- During the summer of 2008 the network was measured once again using a Leica TC2002.
- Four distances were not measured due to line of sight problems.
- All the distances were measured twice from each edge of the line.

Measurements Procedure

- Precise optical plummets were used throughout the campaigns to set up the instrument and prisms over the benchmarks.
- Meteorological conditions were monitored in detail in the line ends during the measurement process.
- The measured distances for each campaign were corrected for the actual meteorological conditions and were reduced to horizontal distances at sea level.

Two Steps Deformation Analysis

Two types of models are pertinent in deformation analysis:

- Mathematical model -
representing the geodetic measurements
- Deformation model –
describe the physical reality

The first step is a rigid adjustment of each campaign to a network of points.

In the second step the variation in network geometry is modeled.

First Step:

Epoch by Epoch Data Processing

- For every measuring campaign the distances were adjusted into a network.
- The variance of the observations was modeled by:
ME5000 → $\sigma_i^2 = (0.5\text{mm})^2 + (d_i \times 1\text{ppm})^2$
TC2002 → $\sigma_i^2 = (1\text{mm})^2 + (d_i \times 1\text{ppm})^2$
where d_i denotes the horizontal distance.

The Kfar-Hanassi network adjustment

| Monitoring Epoch | Instrument | Distances rejected | Degrees of freedom | Mean positional error (1 sigma) | \hat{m}_0 |
|------------------|------------|--------------------|--------------------|---------------------------------|-------------|
| 1990.417 | ME5000 | 1-3, 2-6 3-10 | 41 | 0.58mm | 0.74 |
| 1991.417 | ME5000 | - | 44 | 0.46mm | 0.64 |
| 1992.417 | ME5000 | - | 44 | 0.47mm | 0.64 |
| 1993.500 | ME5000 | 9-12 | 43 | 0.54mm | 0.73 |
| 2008.667 | TC2002 | 5-8 | 40 | 0.61mm | 0.69 |

- The inner accuracy of the measurements is better than the one that was modeled.
- The network accuracy of the campaigns measured with the ME5000 is just slightly better than the one measured with the TC2002.

Second Step:

Movements Analysis

A linear model of movements was tested to describe the plane position of the network benchmarks:

$$x_i = x_0 + \dot{x}\Delta t$$

x_i - Position of a point in time t

t_0 - Reference epoch

\dot{x} - Linear velocity

x_0 - Position at reference epoch

$\Delta t = t - t_0$

Datum Definition

- Distances can be assumed to have scale, the size of the datum defect is 3, since the definition of origin and rotation is missing.
- For the first step we may use the datum parameters that are contained in the distance measurements for estimating the coordinates of the points, but not for estimating the velocities in the deformation analysis.
- Velocities are estimated based on a time series of monitoring campaigns. Therefore, when calculating velocities we should not reduce the datum defects to 3 due to the distances measurements but rather assume a datum defect of 4 parameters.

Statistical Tests of Hypothesis

Statistical tests are applied for estimating the global and partial congruency of the motion model and the significance of its parameters.

$H_0: \hat{\mathbf{x}} = 0$ Accepted if: $w < F_{r,h,\alpha}$

$H_1: \hat{\mathbf{x}} \neq 0$ Accepted if: $w > F_{r,h,\alpha}$

$$w = \frac{\hat{\mathbf{x}}^T \mathbf{Q}_{\hat{\mathbf{x}}}^+ \hat{\mathbf{x}}}{r \hat{m}_0^2} \sim F_{r,h}$$

r- degrees of freedom, h- rank of Q, α - significance level

Results - degrees of freedom

The redundancy of the first step provided 212 degrees of freedom.

The second step provided 72 degrees of freedom,

$$((12 \text{ points} \times 2) \times 5 \text{ campaigns} - (12 \text{ points} \times 2) \times 2)$$

In total 284 degrees of freedom

Results - global congruency test

$$H_0: \hat{x} = 0$$

$$H_1: \hat{x} \neq 0$$

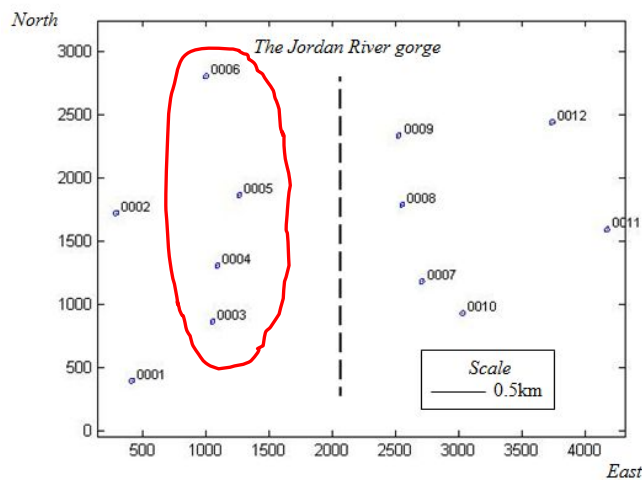
$\alpha=5\%$, $d=4$, $h=20$, $r=284$ then $F(5,20,284) = 1.61$

$$w = 9.62$$

Since $w > F$ the null hypothesis is rejected, indicating that movements occurred during the campaigns.

Results – stable points

4 stable benchmarks 3, 4, 5 and 6, in the west bank of the Jordan River gorge were selected to define the datum.



Results – stable points

$\alpha = 5\%$, $d=4$, $h=4$, $r=284$ then $F(5, 4, 284) = 2.40$

$$w = 1.21$$

Since $w < F$ the null hypothesis is accepted, verifying that the four datum benchmarks were stable.

Results – the weight-constraint solution

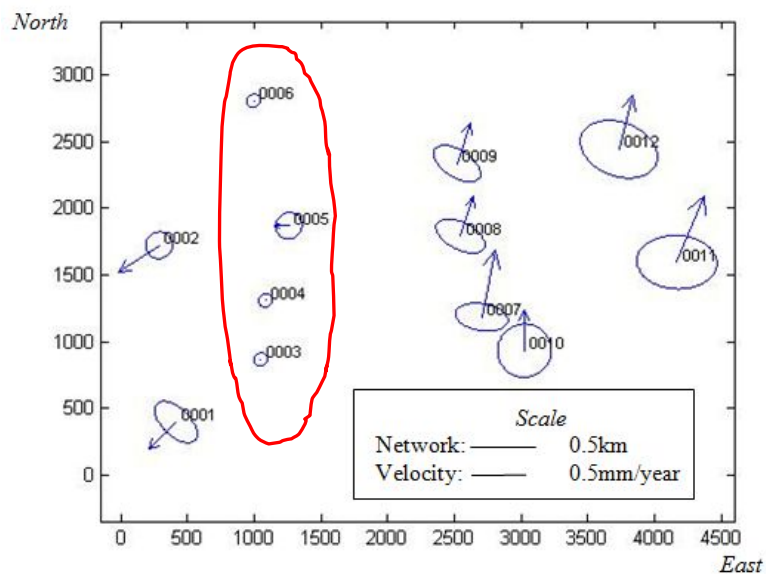
| BM name | \dot{x}_{east} [mm/year] | $\sigma_{\dot{x}_{\text{east}}}$ [mm/year] | \dot{x}_{north} [mm/year] | $\sigma_{\dot{x}_{\text{north}}}$ [mm/year] |
|---------|--------------------------------------|---|---------------------------------------|--|
| 0001 | -0.22 | 0.05 | -0.21 | 0.04 |
| 0002 | -0.29 | 0.03 | -0.23 | 0.04 |
| 0003 | 0.04 | 0.04 | 0.01 | 0.07 |
| 0004 | 0.00 | 0.03 | 0.00 | 0.04 |
| 0005 | -0.06 | 0.04 | -0.03 | 0.04 |
| 0006 | 0.03 | 0.05 | 0.02 | 0.09 |
| 0007 | 0.07 | 0.03 | 0.47 | 0.04 |
| 0008 | 0.09 | 0.04 | 0.34 | 0.04 |
| 0009 | 0.12 | 0.03 | 0.27 | 0.04 |
| 0010 | -0.01 | 0.04 | 0.26 | 0.05 |
| 0011 | 0.16 | 0.04 | 0.55 | 0.06 |
| 0012 | 0.12 | 0.04 | 0.42 | 0.05 |

Results – the single point tests

| BM name | h | r | $F(\alpha, h, r)$ | w | Significant |
|---------|---|--------------|-------------------|-------|-------------|
| 0001 | 2 | 284 | 3.03 | 18.14 | yes |
| 0002 | 2 | 284 | 3.03 | 22.52 | yes |
| 0007 | 2 | 284 | 3.03 | 17.81 | yes |
| 0008 | 2 | 284 | 3.03 | 13.50 | yes |
| 0009 | 2 | 284 | 3.03 | 9.91 | yes |
| 0010 | 2 | 284 | 3.03 | 3.93 | yes |
| 0011 | 2 | 284 | 3.03 | 7.79 | yes |
| 0012 | 2 | 284 <td 3.03 | 7.18 | yes | |

All benchmarks were found to have significantly moved relative to the reference benchmarks.

Results – the velocity field of the Kfar-Hanassi network



Discussion and Conclusions

- Based on the first four campaigns (between 1990 and 1993) no significant movement along the DSR or deformation were detected.
- The large time interval between the four campaigns measured between 1990 and 1993 and the one measured in 2008 dramatically increased the ability of the Kfar-Hanassi network to detect movements.
- Five distance measurement campaigns along a period of 18 years provide us with a very accurate description of current crustal movements in the Kfar-Hanassi region.

Discussion and Conclusions

- The mean velocity of the benchmarks located on the east bank of the Jordan River gorge relative to the datum points, located on the west bank of the river, is **0.38mm/year** in the north direction and **0.09mm/year** in the east direction.
- Benchmarks 1 and 2 which are located on the east bank (as the datum points) move at a rate of **0.22mm/year** in the south-east direction.
- The results indicate a consistent trend of left-lateral motion along the DSR.

Discussion and Conclusions

- The results may indicate that the DSR behaves as a locked fault.
- Additionally to the linear motion model, the locked fault model can be used to describe the plane position of the network benchmarks.